Moment Distribution Method

Lesson Objectives:

- Identify the formulation and sign conventions associated with the Moment Distribution Method.
- 2) Derive the Moment Distribution Method equations using mechanics and mathematics.
- 3) Outline procedure and compute the structural response via Moment Distribution Method.

Background Reading:

1) Read_____

Moment Distribution Method Overview:

1)	This m	nethod was first introduced in	by						
	for the	e analysis of							
2)		s another classical formulation of the							
3)	This m	his method only considers							
		Therefore the assumption is made that _							
		are negligible.							
	b.	Reasonable?							
4)		noment distribution method is useful in the							
	a.	Gain in	to the structural						
		and							
	b.	Does not require to solve a							
		i. This is required within the							
	c.	Useful method to check	·						

Moment Distribution Method Basics:

1)	In ord	er to solve the structure's system of	equations
	simult	aneously:	
	a.	The	is examined at a single joint.
	b.	This is performed in an	
2)		sistent sign convention is established where:	
	a.		are positive when
	b.		are positive when
3)	To per	form the	
	mome	nt distributes at a	
		The term	
		this is a function of the	of members that
		frame the joints.	
	b.	Generally this is written as:	

Derivation of Member Stiffness Values:

1)	Let's consider two different member fixities,	and
	to derive the member stiffness values.	
2)	Recall from the slope-deflection notes, a	
	is defined as the moment developed at	for an
	applied external moment	
	a. For pinned-fixed, the is:	
	b. For pinned-hinged, the is:	

3) Sketch of example member AB where the ends are _____:

4) Curvature diagram sketch of example member AB:

- 5) Assuming that the member is ______ (note _____ is constant), the moment area theorems can be applied.
- 6) Since the rotation at end B is ______(____), the tangential deviation (denoted as ____) is ______.
- 7) Writing the second moment area theorem:

8) Solving for ______, one can find the relationship to the ______:

9) Since ______ is horizontal, one can apply the first moment area theorem as:

a. Relates the applied end moment and rotation of the corresponding end.

10) Now, lets consider the second type of member. A sketch of the second example member

AB where the ends are _____:

11) Curvature diagram sketch of example member AB:

12) From examination of the elastic curve, the rotation at the near end can be written as:

13) Assuming the member is prismatic, the second moment area theorem can be applied as:

- 14) Now with the two aforementioned relationships noted (equations ______ and _____), let's summarize the bending stiffness values.
- 15) Let _____ be defined as the bending stiffness of a member.
 - a. The ______ required at one end of the member to cause a ______

 _______ at that (same) end).
- 16) For a _____ member, this bending stiffness can be expressed as:

17) If ______ is constant, a newly defined ______ (denoted as ____) is:

18) Likewise for a	member:
-	

- a. The bending stiffness is:
- b. The relative stiffness is:

19) To summarize for the two member types (where the ______ varies):

- a. The relative stiffness is:
- b. The moment at the far end, _____, is:
- c. The carryover moment is:

Distribution Factors Introduction and Derivation:

_____:

- - a. This is commonly denoted as _____.
- 2) Sketch of a simple three-member frame structure with an externally _____

3) The free body diagram (FBD) can be drawn as:

4) The free body diagram (FBD) of joint B is:

5) Writing the ______ equilibrium equation:

6) Note that for the three members:

a. Member _____ is ______.

- b. Member _____ is ______.
- c. Member _____ is _____.
- 7) With previous knowledge of the carryover moment, three expressions for end moments can be written as:

8) Substitution of the three equations above (equation _____, and ____) into the equilibrium equation, one can write:

- 9) Let's introduce the terminology of a rotational stiffness.
 - a. This is defined as the _____ required to cause a ______
 ______ at the joint of interest.
 - b. For the example able, the rotation stiffness of joint B is denoted as: _____.
- 10) Some rearrangement of terms will allow for an expression of the unit rotation as:

11) Substitution into the previous equations (equation _____, and ____), expressions can be written as:

12) This can be generalized with the definition of a ______.

- a. A distribution factor is defined as the _________at end B of member i.
- b. Denoted and written in basic form as:

Fixed-End Moments:

- 1) Just as used in the ______, expressions for fixed-end are also required for the moment distribution method.
- 2) Unlike the ______, the effects due to ______ and _____ must be accounted for using FEM.
- 3) Example of FEM due to weak foundations/support settlements:

Procedure for Moment Distribution Method:

- 1) Calculate the distribution factors (_____). Check that ______ at each joint.
- 3) Balance the moments at each joint that is free to rotate in an iterative approach:
 - a. At each joint: first evaluate the ______ and _____ distribute to each member using the ______.
 - b. Perform the _____
 - c. Repeat as required until ______.
- 4) Determine the final member end moments by the sum of ______
 - and _____ moments.
 - a. Note that moment equilibrium must be satisfied for joints that are
- 5) Compute the member end shears by ______.

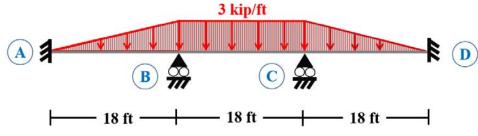
6) Compute the support reactions at joints using ______.

- 7) Check the calculations of the end shears and support reactions using equilibrium.
- 8) Draw the shear and bending moment diagrams, if required.

Moment Distribution Method: Example #1

Problem statement:

Determine the member end moments for the three-span continuous beam illustrated below using the moment-distribution method.



Additional information:

Joints A and D are fixed (moment restrained) Joints B and C are considered as rollers (vertical translation restrained) $L_{AB} = L_{BC} = L_{CD} = 18 \ feet$ $EI = constant \ for \ all \ members$ Note: This beam was previously solved as the first slope-deflection example.

Solution:

1) Determine the distribution factors at joints B and C.

 $DF_{BA} =$

 $DF_{BC} =$

 $DF_{CB} =$

 $DF_{CD} =$

Check distribution factors at each joint. $\Sigma DF_B = DF_{BA} + DF_{BC} =$

 $\Sigma DF_C = DF_{CB} + DF_{CD} =$

2) Compute Fixed-End Moments.

 $FEM_{AB} =$ $FEM_{BA} =$ $FEM_{BC} =$ $FEM_{CB} =$ $FEM_{CD} =$

 $FEM_{DC} =$

 Balance moments at joints and determine final end moments. Begin the moment distribution process by balancing joints B and C.

At joint B:

 $M_B^{unbalanced} = FEM_{BA} + FEM_{BC} =$

 $DM_{BA} = DF_{BA}(-M_B^{unbalanced}) =$

 $DM_{BC} = DF_{BC}(-M_B^{unbalanced}) =$

At joint C:

 $M_{C}^{unbalanced} = FEM_{CB} + FEM_{CD} =$

$$DM_{CB} = DF_{CB}(-M_{C}^{unbalanced}) =$$

$$DM_{CD} = DF_{CD}(-M_C^{unbalanced}) =$$

Perform the carryover moments at the far ends of each beam segment. *Due to the distributed moments at joint B:*

$$COM_{AB} = \frac{1}{2}(DM_{BA}) =$$

$$COM_{CB} = \frac{1}{2}(DM_{BC}) =$$

Due to the distributed moments at joint C:

$$COM_{BC} = \frac{1}{2}(DM_{CB}) =$$

$$COM_{DC} = \frac{1}{2}(DM_{CD}) =$$

Now repeat until the unbalanced moments are negligibly small. It is simple to perform this task in a tabular form (reference Table 1):

Moments	MAB	MBA	M _{BC}	Мсв	MCD	M _{DC}
Distribution Factors						
Fixed-End Moments						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Final Moments						

Table 1. Moment Distribution Table.

- 4) Final end moments are obtained by summing the columns of the moment distribution table (see table above).
- 5) The member end shears and support reactions are determined via equilibrium.

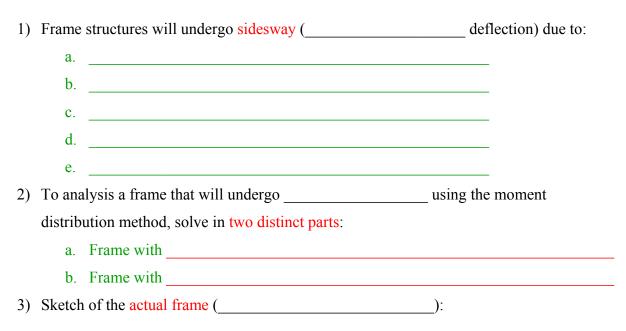
Special Cases of the Moment Distribution Method:

- 1) Simple support at one end.
 - a. Sketch:

- b. Recall that the bending stiffness is:
- c. At the simple support, the distribution factor is _____.
- d. To apply the moment distribution method, balance the joint only ______.
 Leave the free end ______, where the moment is ______.
- 2) Cantilever overhang at one end.
 - a. Sketch:

- b. Section _____ does not contribute to the rotation stiffness at joint _____.
- c. At the free end, the distribution factor is _____.
- d. However, the loads on the cantilever must be still considered at joint _____.

Analysis of Frames with Sidesway:



4) Sketch of the frame with _____

(_____):

5) Sketch of the frame with _____

(_____):

6) Sketch of the frame with _____

(_____):

Procedure for Moment Distribution Method for Frames with Sidesway:

- 1) First solve the frame with external loads for ______
 - a. Find _____.
- 2) Find the fictitious reaction (______) by equilibrium.
- 3) Analyze a second frame with the fictitious reaction applied in the opposite direction.
- 4) Superimpose the relationship of:
- 5) However, one cannot directly compute the values of _____. This requires an in-direct approach.
- 6) Analyze a third frame structure with an unknown translation (______), under an externally applied load of unknown magnitude _____.
 - a. Note _____ is in the opposite direction of _____.
- 7) To solve easily, assume that "____" corresponds to an FEM. Find the remaining FEM values and perform the moment distribution method.
 - a. Find ______.
- 8) Find the value of load _____ by equilibrium.
- 9) The developed moments are linearly proportional to the magnitude of the load.
 - a. Therefore find the ratio of:

10) Assemble the frame by superimposing the loads with the equation:

Moment Distribution Method: Example #2 (Frame with Sidesway)

Problem statement:

Determine the member end moments for the frame illustrated below using the moment-distribution method.

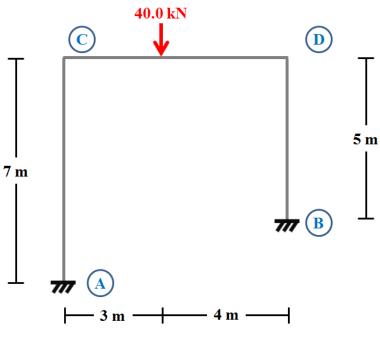


Figure 1. Frame subjected to eccentric loading.

Additional information:

Joints A and B are fixed (moment restrained) Joints C and CD are considered as rigid $L_{AC} = L_{CD} = 7$ meters; $L_{BD} = 5$ meters EI = constant for all members

Solution:

1) Determine the distribution factors at joints C and D.

$$DF_{CA} =$$

 $DF_{CD} =$

 $DF_{DC} =$

 $DF_{DB} =$

Check distribution factors at each joint.

 $\Sigma DF_C = DF_{CA} + DF_{CD} =$

 $\Sigma DF_D = DF_{DC} + DF_{CD} =$

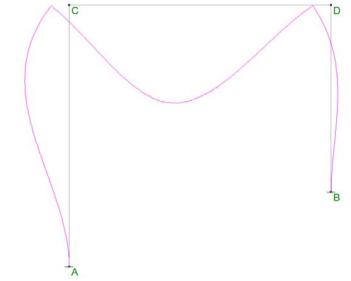


Figure 2. Displaced shape for actual frame with applied loads (from RISA-2D).

2) Frame that is "sidesway prevented":

In the first part of analysis, envision a fictitious roller to prevent ________ at joint C. Assume that the joints C and D are therefore clamped against rotation, compute the fixed-end moments due to the external load.

 $FEM_{CD} =$

 $FEM_{DC} =$

$$FEM_{AC} = FEM_{CA} = FEM_{BD} = FEM_{DB} =$$

For the case with *sidesway translation prevented*, perform moment distribution analysis. This procedure is performed below in tabular form (reference Table 2):

Moments	MAC	MCA	Мср	MDC	MDB	MBD
Distribution Factors						
Fixed-End Moments Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Final Moments						

Table 2. Moment Distribution Table (sidesway prevented).

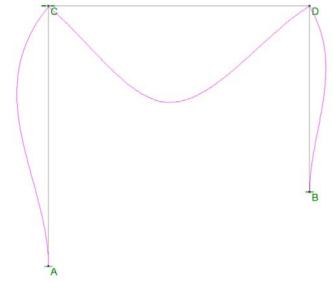
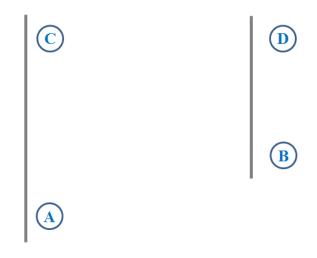
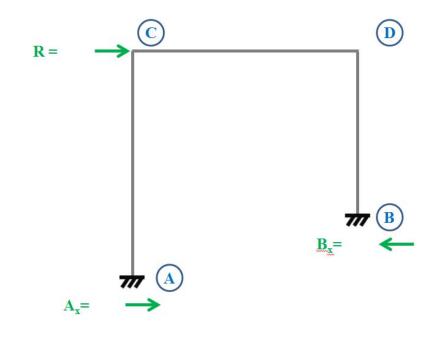


Figure 3. Displaced shape for frame with applied loads where sidesway is prevented (from RISA-2D).

To evaluate the fictitious restraining force which develops at the imaginary roller at C, calculate the end shears and moments by equilibrium.



Then consider equilibrium of the entire frame and compute the reaction at C by evaluating the forces in the horizontal direction.



 $\Sigma F_x = 0$:

 $R_x =$

Note: the computed restraining force acts to the right, indicates that if the roller support was not present, the frame would displace to the left direction.

3) Frame that is "sideway permitted":

The actual frame is not restrained in the lateral direction, therefore one will negate its effect by applying a lateral load to a second frame in the opposite direction.

Recall from the class notes, the moment distribution method cannot directly compute the end moments due to the lateral load. Therefore one will employ an indirect approach where the frame is subjected to an unknown joint displacement, Δ '. Note Δ ' is caused by an unknown force, Q, acting at the same location and opposite direction of R (fictitious roller reaction).

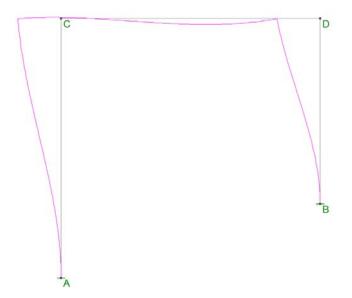


Figure 3. Displaced shape for frame subjected to an arbitrary translation (from RISA-2D).

Assume the joints C and D are clamped against rotation, compute the fixed-end moments due to the translation, Δ '.

 $FEM_{AC} = (-)\frac{6EI\Delta'}{L^2} =$ $FEM_{CA} = (-)\frac{6EI\Delta'}{L^2} =$ $FEM_{BD} = (-)\frac{6EI\Delta'}{L^2} =$ $FEM_{DB} = (-)\frac{6EI\Delta'}{L^2} =$ $FEM_{CD} = FEM_{DC} =$

Negative signs indicate that these moments are _____

In lieu of computing the FEM for the moment distribution method as a function of Δ ', arbitrarily assume one fixed end moment.

 $FEM_{AC} =$

Solve for Δ ' and evaluate values for the remaining fixed end moments using substitution.

 $\Delta' =$

$$FEM_{AC} = (-)\frac{6EI\Delta'}{L^2} =$$

$$FEM_{CA} = (-)\frac{6EI\Delta'}{L^2} =$$

$$FEM_{BD} = (-)\frac{6EI\Delta'}{L^2} =$$

$$FEM_{DB} = (-)\frac{6EI\Delta'}{L^2} =$$

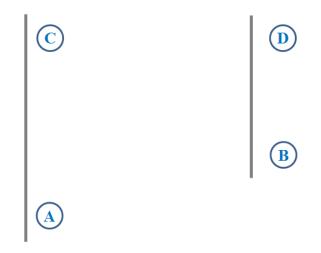
$$FEM_{CD} = FEM_{DC} =$$

Now the case with frame which undergoes lateral translation is analyzed. Perform moment distribution analysis in the traditional manner, shown here in tabular form (reference Table 3):

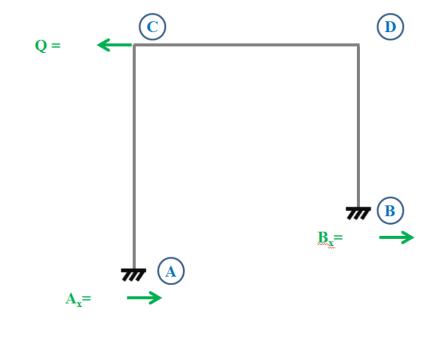
Moments	MAC	MCA	Мср	MDC	Mdb	M _{BD}
Distribution Factors						
Fixed-End Moments Balance						
Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints						
Carryover						
Balance Joints					-	
Carryover						
Balance Joints						
Final Moments						

Table 3. Moment Distribution Table (<u>unknown lateral translation due to Q</u>).

To evaluate the magnitude of Q which resulted in a lateral translation of Δ ', calculate the end shears and moments by equilibrium.



Then consider equilibrium of the entire frame and compute the value of Q by evaluating the forces in the horizontal direction.



$$\Sigma F_{x} = 0$$
:

Q =

Therefore the moments calculated in the second moment distribution method are caused by the lateral force Q. Recall that the <u>moments developed are linearly proportional to the</u> <u>to the magnitude of the load</u>, the desired moments corresponding to the fictitious restraint reaction are then multiplied by the ratio of R/Q.

$$\frac{R}{Q} =$$

Actual member end moments by superposition.
 Now the actual member end moments can be determined by summing the moments computed due to each moment distribution method.

$$M = M_o + M_R$$
$$M_r = \left[\frac{R}{Q}\right] M_q$$

where:

 $M_o = moments$ created due to external loads where sidesway is prevented $M_r = moments$ created due to fictitious reaction (opposite direction) $M_q = moments$ created due to an arbitrary translation

Computing end moments:

$$M_{AC} = M_{o_{AC}} + \left[\frac{R}{Q}\right] M_{Q_{AC}} =$$

$$M_{CA} = M_{o_{CA}} + \left[\frac{R}{Q}\right] M_{Q_{CA}} =$$

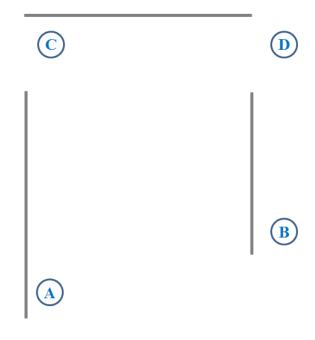
$$M_{CD} = M_{o_{CD}} + \left[\frac{R}{Q}\right] M_{Q_{CD}} =$$

$$M_{DC} = M_{o_{DC}} + \left[\frac{R}{Q}\right] M_{Q_{DC}} =$$

$$M_{DB} = M_{o_{DB}} + \left[\frac{R}{Q}\right] M_{Q_{DB}} =$$

$$M_{BD} = M_{o_{BD}} + \left[\frac{R}{Q}\right] M_{Q_{BD}} =$$

5) The actual member end shears and support reactions are determined via equilibrium.



$$A_x = B_x =$$

$$A_y = B_y =$$

 $M_A = M_B =$