Slope Deflection Method

Lesson Objectives:

- 1) Identify the formulation and sign conventions associated with the Slope Deflection method.
- 2) Derive the Slope Deflection Method equations using mechanics and mathematics.
- 3) Describe the concept of fixed-end moments.
- 4) Outline procedure and compute the structural response using the Slope Deflection Method.

Ü	round Reading:	
1)	Read	
lope]	Deflection Method Overview:	
1)	This method was first introduced in	by
	for the analysis of	
2)		
		analysis method that will be detailed at a later
	time.	
4)	This method only considers	
		de that
	are negligible.	
	b. Reasonable?	
5)		are and
	they are solved initially using	
		and

Slope Deflection Fundamental Relationships:

1)	When the structure of interest, (_ or)
	is subjected to external loads,			_ develop a
	the individual member ends.			
2)	The primary goal of the slope defle	ection equation is to relate th	e	
	to the	and		_ at its ends
	along with the			
3)	For derivation, let's consider a bea			
	is a cut section from a continu	ous beam.		
Ā	Figure 1 For			
	Figure 1. Ex	ample continuous beam ¹ .		
4)	A consistent sign convention is est a.	ablished where:		
		, and		
	are positive when			
5)	The continuous beam section.			

the member ends.

6) As the member deforms, _____ are induced at

(Figure 2).

¹ All figures in Slope Deflection were modified from: Kassimali, Aslam. (2014). *Structural Analysis*. 5nd edition. Cengage Learning.

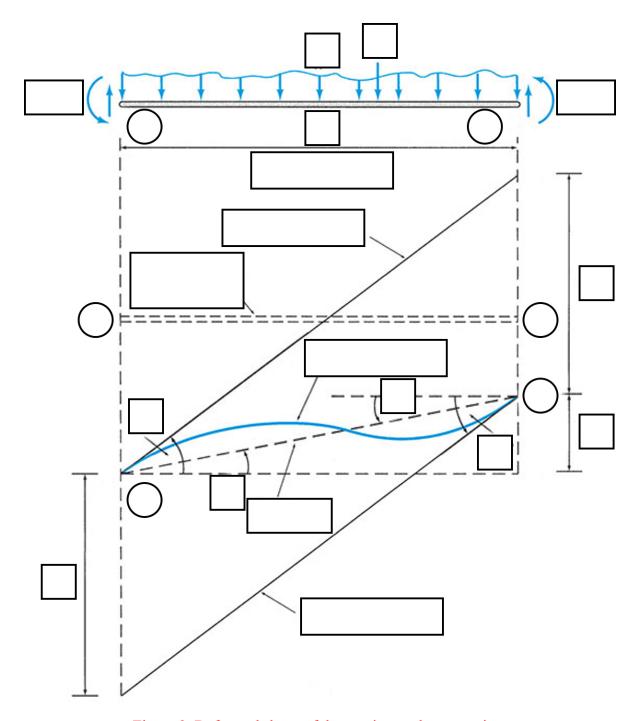


Figure 2. Deformed shape of the continuous beam section.

Notation within the Slope-Deflection Method:

1)	The typ	pical notation	used within the slope	deflection n	nethod to denote an en	nd moment:
	a.	$M_{AB} = \text{mome}$	ent at the end of	of meml	ber	
	b.	Alternatively	: A can refer to		, while B denot	es the
2)			of a member with formed position for a	•	-	
3)	The rel	lative	•	be	tween two ends of the	e member in a
	denote	d as:				
4)	The ro	tation of the m	nember's chord is deno	oted as:	_ (pronounced as).
	a.	A chord is the	e		that connects the	
			du	e to relative	e translation ().	
	b.	If small defor	rmations are assumed,	geometrica	lly this can be expres	sed as:
Slope-	Deflect	ion Derivation	n:			
1)	Deriva	tion of the slop	pe-deflection equation	ns can perfo	rmed using the	
2)	This ed	quation can be	written as:	•		

3)	From the geometry shown in Figure 2, two equations can be written for the rotation at each end:					
4)	Substitution in the known relationship of					
5)	Recall that the tangential deviations are specifically detailed as:					
3)						
	a denotes the tangential deviation of end from the tangent to the elastic curve at end					
	b denotes the tangential deviation of end from the tangent to the					
	elastic curve at end .					
6)	These tangential deviations can be found using the					
0)						
	a. How?					
7)	To construct the required bending moment diagram, apply,,					
	and the separately over the beam section with					
	a. This will create a simple bending moment					
	diagram as illustrated in Figure 3.					

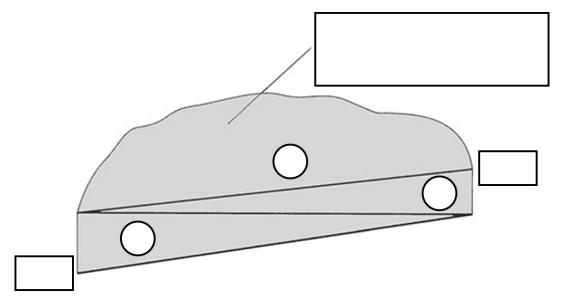


Figure 3. Illustration of the bending moment diagram along the continuous beam section.

8)	Within the derivation, another assumption is made that the member section is						
	(where is constant). This allows for a simple						
	application of the						
	a. Note the required tangential deviations are illustrated in Figure 4.						
9)	In Figure 4, and denote moments about the ends and,						
	respectively, of the area under the simple beam bending moment diagram.						
10) In this same figure, denote an arbitrary						

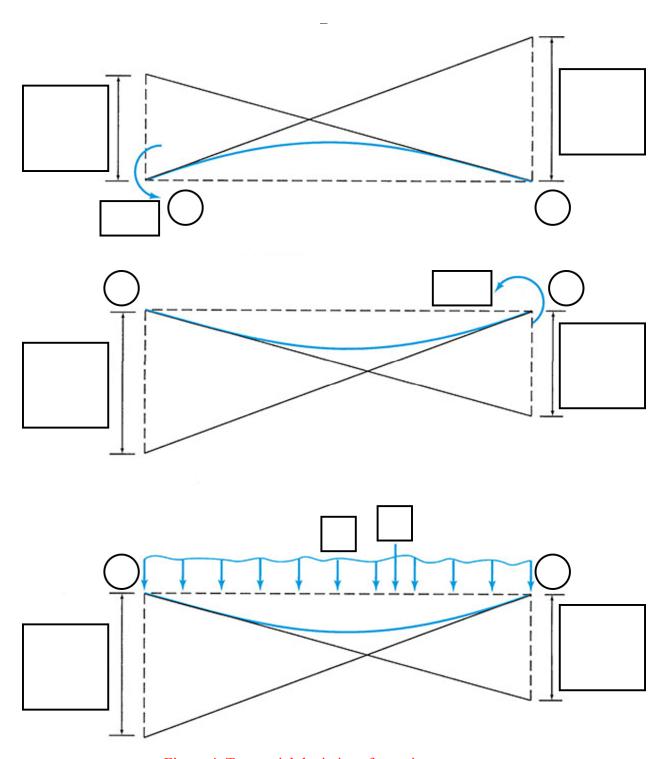


Figure 4. Tangential deviations for various moments.

Slope-Deflection Derivation Continued:

	Using the second moment area theorem, expressions for each of the tangential deviation
	can be written as:
•	
2)	Note the tangential deviations due to, and are
2)	Note the tangential deviations due to, and are indicated on Figure 4.
	indicated on Figure 4.
3)	indicated on Figure 4. In this figure, the negative terms indicate tangential deviations in the to the elastic curve.
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	6)	Substitution of equa	ation into	, allows one to find	
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7)	Substitution of eq	uation	into	, allows o	one to find	
''	Daobitation of eq	aution	11100	, 4110 115 1	one to mind	

- 8) These two previous equations (_____ and _____) indicate that the moments that develop at the member ends depend on:
 - a. _____ of the member ends.
 - b. _____ of the member ends.
 - c. ______ between member ends.
- 9) Now let's assume that the member considered is part of a larger structure, where the isolated beam has
 - a. Illustrated in Figure 5.
 - b. Note that the _____ and ____ of each end is ____ .

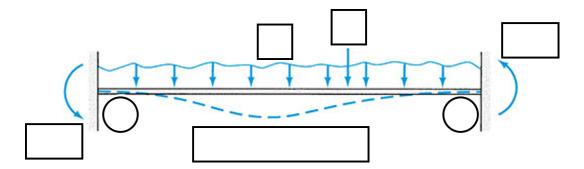


Figure 5. Fixed-end moments (FEM)

10) Using	equations	and	_ as well as thro	ugh setting,	, and
	, expressions	can be writte	en for the		;
1) Where		= the	e fixed-end mome	ent due to	
				previous set after t	
			and).	•	
) into the previo	us equation set
	_ and):	1			1
4) These	equations abo	ove (a	nd) expres	s the moment of th	e member ends in
5) The af	Corementioned	equations (and)	represent the	
				. They are only va	alid when:
a.	The member	considered	is		<u></u> .
b.	The member	does not co	ntain a		<u></u> .
c.	The material	behavior is			
d.					
e.	Characterize	deformation	n only under		
	therefore			are i	neglected.

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16)	This can be written in a single generalized equation as:
Fixed-1	End Moments:
	Expressions for fixed-end moments can be derived.
	However, it is very common to find FEM expressions in a tabular form.
3)	Resources typically include:
	a
	b
	c
	d
	Two example tables (for simplistic externally applied loads) that identify moment
,	diagrams, maximum deflections, and fixed-end moments are included here for reference.

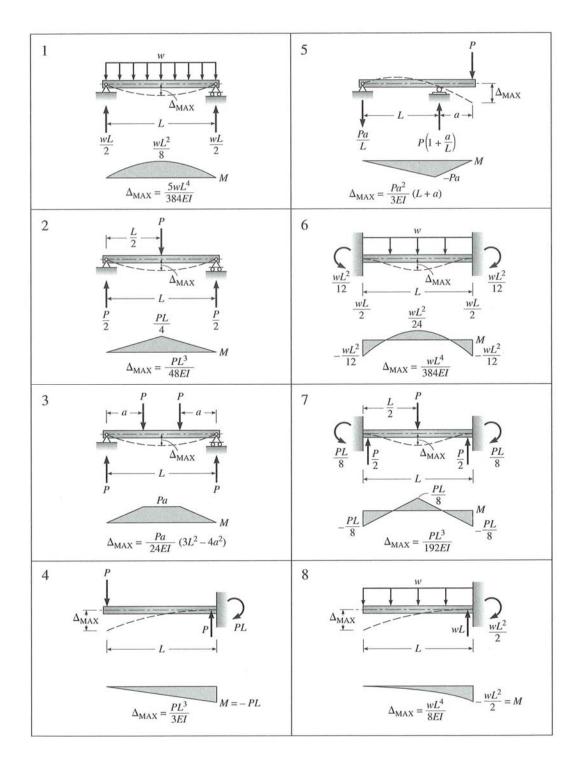


Figure 6. Moment diagrams and equations for the maximum deflection. (Obtained from: Leet, K.M., Uang, C.M., and Gilbert, A.M. (2011) *Fundamentals of Structural Analysis*. 4th Edition. McGraw Hill, New York).

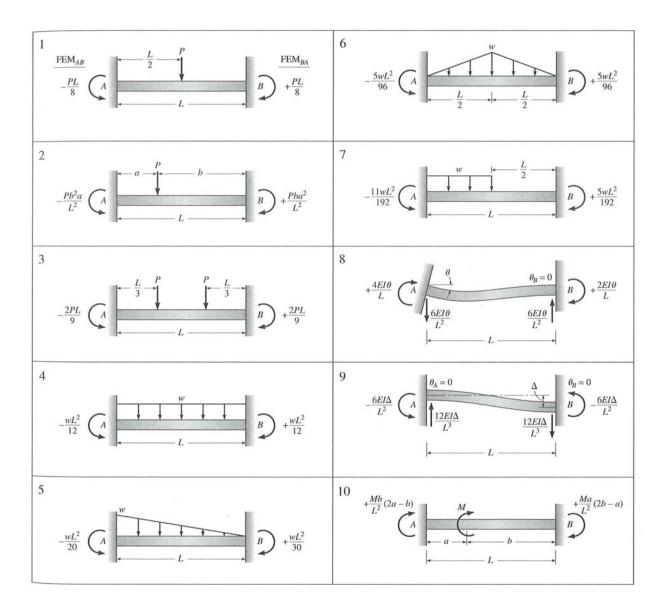


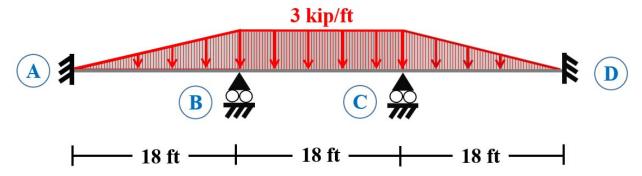
Figure 7. Fixed-End Moments. (Obtained from: Leet, K.M., Uang, C.M., and Gilbert, A.M. (2011) *Fundamentals of Structural Analysis*. 4th Edition. McGraw Hill, New York).

Procedure for Slope-Deflection:

1)	Identify the degrees of freedom (DOFs).	
	a. For a continuous beam, this is only	 -
	b. For a frame structure, this may include	and
2)	Compute the fixed-end moment (FEM) for each member.	
	a. Use handout or other available resources.	
3)	If support settlement exists, compute the	
	or	
	a. Preferences?	_
4)	Write the slope-deflection equations for each member.	
5)	Write the equilibrium equations.	
6)	Determine the deformation at the joints (or member ends) by	
7)	Calculate the member end moments.	
8)	Check for equilibrium equations.	
	a. Equations written in step	
9)	Compute the member end shears by	<u>.</u>
10	Compute the support reactions at joints using	
	Check the calculations of the end shears and support reactions us	
12	Draw the shear and bending moment diagrams, if required.	

Slope Deflection Method: Example #1

Compute the reactions and draw the moment and shear diagrams for the three-span continuous beam as illustrated. Use the slope-deflection method.



- 1) Identify the degrees of freedom:
- 2) Calculate the fixed-end moments and chord rotations:

3)	Substit	tute into the slope-deflection equations:			
	a.	The slope-deflection equation is:			
	b.	Note the sign conventions of where:	,		
		, and are			
		positive when			
4)	White 4	the equilibrium equations.			
4)	write 1	the equilibrium equations:			

5)	Solve	for the joint deformations:
	a.	Substitute slope-deflection equations for M_{BA} , M_{BC} , M_{CB} , and M_{CD} into the equilibrium equations.
6)	Comp	ute the member end moments:
0)	a.	Substitute known values of and into the slope-deflection equations.
7)	Check	equilibrium at joints:



Additional Slope-Deflection Insights:

1)	From the slope deflection method, a few aspects often reappear in other topics a.
	b
2)	One common structural system is a fixed-pinned beam that is rotated at the pinned end. A
	sketch is illustrated here:
3)	The moment applied at the pinned end is:
4)	Therefore the moment at the other end () is:
5)	This lands to a leav observations, the moment applied at one and
3)	This leads to a key observations, the moment applied at one end to at the other end.
6)	This is known as the carry-over moment:
	The carry-over moment is a fundamental relationship used within the
1)	
	·
Modif	ied Slope Deflection Equation:
1)	Previously the slope deflection equation was only valid if the both of the member ends
	are
2)	A modified equation can be used to account for members that are
3)	Sketch of member:

4)	The slope deflection equations can be written and then modified as:
5)	Solve for in equation and then substitute into equation :
6)	Likewise, can do the same derivation for an alternative structure:
7)	This equation would be written as:
8)	Expressing this in a generalized form:

Slope Deflection Method: Example #2

Determine the member end moments and reactions for the frame structure shown below using the slope-deflection method.

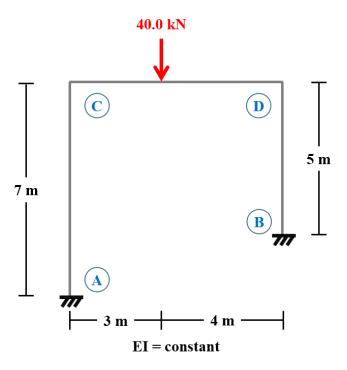


Figure 8. Three member frame for example 2^2 .

1) In this structure, the analyst must account for the following:

2) Identify the degrees of freedom:

² Example from: Kassimali, Aslam (2014). *Structural Analysis*, 5th Edition. Cengage Learning.

3) Draw the deflected shape (qualitatively):

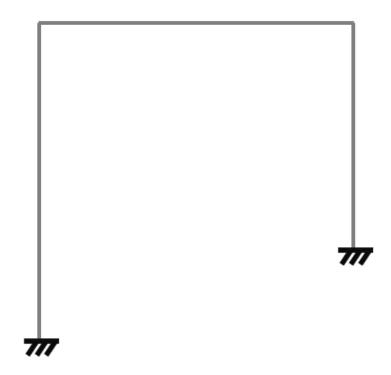


Figure 9. Deflected shaped of frame structure.

4) Calculate the fixed-end moments and chord rotations:

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5) Substitute into the slope-deflection equations:				

6) Write the equilibrium equations:

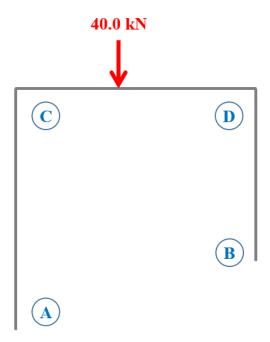


Figure 10. Free body diagram of entire frame structure.

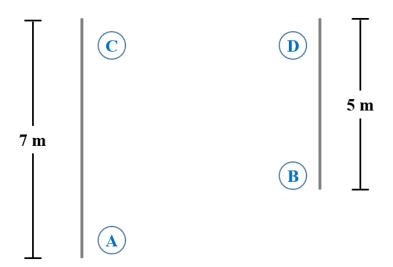


Figure 11. Free body diagrams of the two vertical columns.

7) Solve for the joint deformations:

a. We now have 3 equations and 3 unknowns, therefore solve the system of equations.

Joint Displacements. To determine the unknown joint displacements θ_C , θ_D , and Δ , we substitute the slope-deflection equations (Eqs. (1) through (6)) into the equilibrium equations (Eqs. (7) through (9)) to obtain

$$1.142EI\theta_C + 0.286EI\theta_D + 0.122EI\Delta = -39.2$$
 (10)

$$0.286EI\theta_C + 1.371EI\theta_D + 0.24EI\Delta = 29.4$$
 (11)

$$4.285EI\theta_C + 8.4EI\theta_D + 4.58EI\Delta = 0$$
 (12)

Solving Eqs. (10) through (12) simultaneously yields

$$EI\theta_C = -40.211 \text{ kN} \cdot \text{m}^2$$

$$EI\theta_D = 34.24 \text{ kN} \cdot \text{m}^2$$

$$EI\Delta = -25.177 \text{ kN} \cdot \text{m}^3$$

8) Compute the member end moments and complete the problem:

Member End Moments. By substituting the numerical values of $EI\theta_C$, $EI\theta_D$, and $EI\Delta$ into the slope-deflection equations (Eqs. (1) through (6)), we obtain

$$M_{AC} = -14.6 \text{ kN} \cdot \text{m}$$
 or $14.6 \text{ kN} \cdot \text{m}$ Ans.

$$M_{CA} = -26 \text{ kN} \cdot \text{m}$$
 or $26 \text{ kN} \cdot \text{m}$)

$$M_{BD} = 7.7 \text{ kN} \cdot \text{m}$$
 Ans.

$$M_{DB} = 21.3 \text{ kN} \cdot \text{m}^{5}$$

$$M_{CD} = 26 \text{ kN} \cdot \text{m}^{5}$$

$$M_{DC} = -21.3 \text{ kN} \cdot \text{m}$$
 or $21.3 \text{ kN} \cdot \text{m}$)

To check that the solution of the simultaneous equations (Eqs. (10) through (12)) has been carried out correctly, we substitute the numerical values of member end moments back into the equilibrium equations (Eqs. (7) through (9)):

$$M_{CA} + M_{CD} = -26 + 26 = 0$$
 Checks

$$M_{DB} + M_{DC} = 21.3 - 21.3 = 0$$
 Checks

$$5(M_{AC} + M_{CA}) + 7(M_{BD} + M_{DB}) = 5(-14.6 - 26) + 7(7.7 + 21.3) = 0$$
 Checks

Member End Shears. The member end shears, obtained by considering the equilibrium of each member, are shown in Fig. 15.17(e).

Member Axial Forces. With end shears known, member axial forces can now be evaluated by considering the equilibrium of joints C and D. The axial forces thus obtained are shown in Fig. 15.17(e).

Support Reactions. See Fig. 15.17(f).

Ans.

Equilibrium Check. The equilibrium equations check.

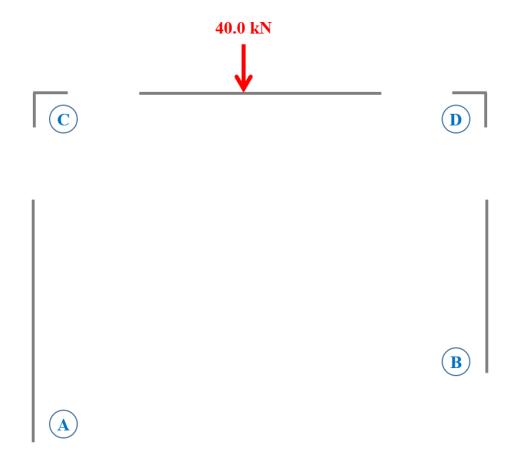


Figure 12. Frame structure: member end moments, shears, and axial forces.

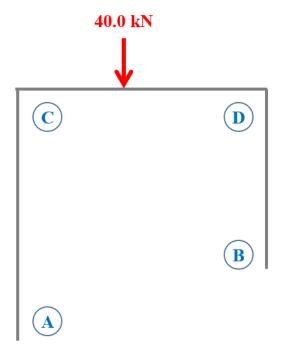


Figure 13. Frame structure: support reactions.